## TRAFFIC IMPACT STUDY

For

# Stack Storage, LLC Proposed Self-Storage Facility

Property Located at:

Crine Road (CR 4)/Vanderburg Road and Boundary Road Block 360 – Lots 7 & 8 Township of Marlboro, Monmouth County, NJ



1904 Main Street | 245 Main Street, Suite #110 Lake Como, NJ 07719 | Chester, NJ 07930 (732) 681-0760

Nick Verderese, PE NJ PE License #38991 Justin P. Taylor, PE, PTOE NJ PE License #45988

June 22, 2021

3724-99-001T



### **INTRODUCTION**

It is proposed to construct a self-storage facility on a parcel of land that is currently undeveloped, located in the southwest corner of the intersection of Crine Road (CR 4)/Vanderburg Road and Boundary Road in the Township of Marlboro, Monmouth County, New Jersey (see Figure 1 in Appendix A). The site is designated as Block 360 – Lots 7 and 8 on the Marlboro Township Tax Maps. It is proposed to construct a 89,700 SF self-storage facility ("The Project"). The site is located within the LI - Light Industrial Zone. It is proposed to provide access to The Project via a full movement driveway along Vanderburg Road.

Dynamic Traffic LLC has been retained to prepare this study to assess the traffic impact associated with the construction of The Project on the adjacent roadway network. This study documents the methodology, analyses, findings and conclusions of our study and includes:

- A detailed field inspection was conducted to obtain an inventory of existing roadway geometry, traffic control, and location and geometry of existing driveways and intersections.
- Existing traffic data was collected via manual turning movement (MTM) counts during the weekday PM and Saturday midday peak periods at the intersection of Crine Road (CR 4)/ Vanderburg Road and Boundary Road.
- Projections of traffic to be generated by the proposed development were prepared utilizing trip generation data as published by the Institute of Transportation Engineers. Site traffic was then assigned to the adjacent street system based upon the anticipated directional distribution.
- Capacity analyses were conducted for the Existing, No Build, and Build conditions for the study intersections.
- The proposed point of ingress and egress was inspected for adequacy of geometric design, spacing and/or alignment to streets and driveways on the opposite side of the street, relationship to other driveways adjacent to the development, and conformance with accepted design standards.
- The site plan as designed was reviewed for sufficiency in accommodating large wheel base vehicles such as recreational vehicles or emergency vehicles.
- The parking layout and supply was assessed based on accepted design standards, local requirements, and demand experienced at similar developments.



### **EXISTING CONDITIONS**

A review of the existing roadway conditions near the proposed site was conducted to provide the basis for assessing the traffic impact of the development. This included field investigations of the surrounding roadways and intersections, collection of traffic volume data, and extensive analyses.

### **Existing Roadway Conditions**

The following are descriptions of the roadways in the study area:

<u>Crine Road (CR 4)</u> is an Urban Major Collector roadway under Monmouth County jurisdiction with a general east/west orientation. In the vicinity of the site the posted speed limit is 45 MPH and the roadway provides one travel lane in each direction. On-street parking is not permitted. Curb and sidewalk are not provided along either side of the roadway. Crine Road (CR 4) provides a straight horizontal alignment and an upgrade from east to west. The land uses along Crine Road (CR 4) in the vicinity of The Project are primarily residential.

<u>Vanderburg Road</u> is an Urban Major Collector roadway under Marlboro Township jurisdiction with a general east/west orientation. In the vicinity of the site the posted speed limit is 45 MPH and the roadway provides one travel lane in each direction. On-street parking is not permitted. Curb is provided along both sides of the roadway, while sidewalk is not provided along either side of the roadway. Vanderburg Road provides a straight horizontal alignment along the site frontage and a relatively flat vertical alignment. The land uses along Vanderburg Road in the vicinity of The Project are mixed residential, commercial, and industrial.

Boundary Road is an Urban Major Collector roadway under Colts Neck Township and Marlboro Township jurisdiction with a general north/south orientation. In the vicinity of the site the posted speed limit is 40 MPH and the roadway provides one travel lane in each direction. On-street parking is not permitted. Curb is provided along the northbound side of the roadway to the north of Crine Road (CR 4)/Vanderburg Road and along the southside of the roadway to the south of Crine Road (CR 4)/Vanderburg Road. Sidewalk is not provided along either side of the roadway. Boundary Road provides a relatively straight vertical alignment and a downgrade from north to south. The land uses along Boundary Road in the vicinity of The Project are mixed residential and industrial.

### **Existing Traffic Volumes**

Manual turning movement (MTM) counts were conducted on Thursday, June 10, 2021 from 4:30 to 6:30 PM and on Saturday, June 12, 2021 from 11:00 AM to 2:00 PM at the intersection of Crine Road (CR 4)/Vanderburg Road and Boundary Road.

Review of the collected traffic data reveals that the weekday evening PSH occurs between 4:30 PM - 5:30 PM and the Saturday PSH occurs between 12:45 PM - 1:45 PM. Figure 2, located in Appendix B, shows the existing peak hour traffic volumes at the study intersections. All traffic counts are contained in Appendix B.



### COVID-19 Traffic Count Normalization

It should be noted that various protocols associated with the COVID-19 pandemic were in effect as of the time of the traffic counts. As a result, current traffic volumes on the surrounding roadways may be atypically low at this time and would not be representative of "existing" traffic conditions. While there are currently no historical traffic volumes published on the study roadways in the vicinity of the site, it is noted that this firm recently conducted a traffic count comparison for existing traffic volumes in Marlboro Township, contained within the *Traffic Impact Study* for the Marlboro Community Commerce Park, prepared Dynamic Traffic and dated April 30, 2021. Specially, this firm conducted MTM counts at the intersection of Amboy Road and Tennent Road on Tuesday, April 20, 2021 and normalized these volumes utilizing a June 2018 NJDOT ATR located along Tennant Road just east of Amboy Road.

In order to perform an appropriate comparison, the 2018 NJDOT traffic volumes were increased to better represent existing 2021 traffic volumes by applying a growth rate of 1.0% per year obtained from the NJDOT Annual Background Growth Rate Table for a period of three (3) years. The adjusted 2018 traffic volumes were then compared to the existing 2021 traffic counts as summarized in the table below.

Table I Traffic Count Comparison

		Peak Hour T	Traffic Volume	
Location	Date	As-Counted	With Background Growth [1]	% Difference
		PM	PM	PM
Amboy Road and	June 2018	1,057	1,089	-9%
Tennent Road	April 2021	1,196	1,196	-9%

<sup>[1]</sup> June 2018 data increased by 1.00% per NJDOT Annual Background Growth Rate Table compounded annually for three years.

As seen above, the current traffic volumes were found to be higher than the historical traffic volumes grown to represent existing conditions during the weekday evening peak hour. It is noted that there is no published historical traffic data published during the Saturday peak hour. Therefore, no adjustment factor was applied to peak hour volumes.

### **Existing Capacity Analysis**

The methodology utilized in the capacity analyses is described in the *Highway Capacity Manual*, published by the Transportation Research Board. In general, the term Level of Service (LOS) is used to provide a "qualitative" evaluation of capacity based upon certain "quantitative" calculations related to empirical values, such as traffic volume and intersection control.

An unsignalized (STOP sign controlled) driveway or side street along a through route is seldom critical from an overall capacity standpoint, however, it may be of great significance to the capacity of the minor cross-route, and it may influence the quality of traffic flow on both. When analyzing an unsignalized intersection, it is assumed that both the major street through and right turn movements are unimpeded and have the right-of-way over all side street traffic and left turns from the major street. All other turning movements in the intersection cross, merge with, or are otherwise impeded by major



street movements. Traffic delays at unsignalized intersections are determined by sequentially processing these impeded movements. Table II describes the level of service ranges for unsignalized (stop controlled) intersections.

Table II
Level of Service Criteria
for Unsignalized Intersections

Level of Service	Average Control Delay (seconds per vehicle)
a	0.0 to 10.0
b	10.1 to 15.0
С	15.1 to 25.0
đ	25.1 to 35.0
e	35.1 to 50.0
f	greater than 50.0

All capacity analyses were performed utilizing Synchro 11 software. Table III summarizes the existing levels of service (LOS) and delays. All capacity analysis calculation worksheets are contained in Appendix C.

Table III Existing Levels of Service

Intersection		ction/ ement	PM PSH	SAT PSH
	EB	L	a (8)	a (7)
Crine Road (CR 4)/Vanderburg Road and	WB	L	a (8)	a (7)
Boundary Road	NB	LTR	b (14)	b (11)
	SB	LTR	b (15)	b (11)

a (#) - Unsignalized Intersection Level of Service (seconds of delay per vehicle)

The following are discussions pertaining to each of the existing intersections analyzed. It should be noted that the existing percentage of trucks and peak hour factors were used in the existing analysis.

### Crine Road (CR 4)/Vanderburg Road and Boundary Road

Crine Road (CR 4)/Vanderburg Road intersects Boundary Road to form a four-leg intersection operating with the northbound and southbound approaches of Boundary Road under stop control. The eastbound and westbound approaches of Crine Road (CR 4)/Vanderburg Road both provide a shared left turn/through/right turn lane. The northbound and southbound approaches of Boundary Road both provide a shared left turn/through/right turn lane.

A review of the existing analysis reveals that all movements operate at levels of service "B" or better during the analyzed peak periods. See Table III for the individual movement levels of service and delays.



### **FUTURE CONDITIONS**

Traffic volumes and operational analyses were developed for both the 2023 No Build and Build conditions. The No Build conditions provide a baseline for assessing the impact of the site development traffic on the roadway system. The process of developing the No Build and Build traffic volumes and the subsequent analyses is outlined below.

Regardless of whether the subject site is developed or not, traffic volumes on the surrounding roadways are expected to increase as a result of developments throughout the region. A growth rate for roadways within the study area was obtained from the NJDOT Annual Background Growth Rate Table, which indicates a growth rate of 2.5% per year.

Through consultation with the Marlboro Township Planning Board staff, there is one development in the vicinity of the site that has been approved but not yet constructed that is identified as a potential significant traffic generator, shown below. The Adjacent Development Traffic Volumes passing the site are shown on Figure 3. It was assumed that the background growth rate was adequate to account for the traffic associated with all developments not listed hereafter.

• A development consisting of 85 townhome units, 20 apartment units, a 4,000 SF drive-in bank, and 8,110 SF of retail space known as Marlboro Green, located in the southeast corner of the intersection of NJ Route 79 and Stevenson Drive, has been approved. Projections of the associated traffic volumes were obtained from Figure 12 contained within the Appendix A of the *Traffic Impact Study*, prepared by Langan Engineering and Environmental Services, Inc and dated April 30, 2020.

Future 2023 No Build traffic volumes were developed by applying the background growth rate of 2.5% for two (2) years to the study area roadways existing traffic volumes and adding the adjacent development traffic volumes. Figure 4, in Appendix A, shows the 2023 No Build traffic volumes.

### **Traffic Generation**

Trip generation projections for The Project were prepared utilizing trip generation research data as published under Land Use Code 151 – Mini-Warehouse in the Institute of Transportation Engineers' (ITE) publication, *Trip Generation*, 10<sup>th</sup> Edition. This publication sets forth trip generation rates based on traffic counts conducted at research sites throughout the country.

Table IV
Trip Generation

Trin Tymo		PM PSH	•	9	SAT PSH	[
Trip Type	In	Out	Total	In	Out	Total
89,700 SF Self-Storage Facility	7	8	15	17	11	28

Once the magnitude of traffic to be generated by the site is known, it is necessary to assign that traffic to the adjacent street system. The distribution of new traffic to the surrounding roadways is based on the location of primary arterial roadways, major signalized intersections and existing traffic patterns. Located in Appendix A, Figures 5 and 6 illustrate the Trip Distribution and the Site Generated



Volumes, respectively. The Site Generated Volumes assigned to the study area network were added to the No Build traffic volumes to generate the Build traffic volumes, which are shown in Figure 7.

### **Future Capacity Analysis**

Operational conditions at the study intersections were analyzed under the No Build and Build conditions and are summarized in Table V below.

Table V Future Levels of Service

	Dirac	ction/	PM	PSH	SAT	PSH
Intersection		ement	No Build	Build	No Build	Build
Crima Danid (CD 4) /	EB	L	a (8)	a (8)	a (7)	a (7)
Crine Road (CR 4) /	WB	L	a (8)	a (8)	a (7)	a (8)
Vanderburg Road and Boundary Road	NB	LTR	b (15)	b (15)	b (12)	b (12)
Boulldary Road	SB	LTR	c (16)	c (16)	b (11)	b (12)
Vanderburg Road and Site	WB	L	-	a (8)	-	a (8)
Driveway	NB	LR	-	b (11)	-	a (10)

a (#) - Unsignalized Intersection Level of Service (seconds of delay per vehicle)

### Crine Road (CR 4)/Vanderburg Road and Boundary Road

With the addition of site generated each movement is anticipated to operate at No Build levels of service "C" or better. See Table V for the individual movement levels of service and delays.

### Vanderburg Road and Site Driveway

The site driveway is proposed to intersect Vanderburg Road to form an unsignalized T-intersection with the northbound approach of the site driveway operating under stop control. The eastbound approach of Vanderburg Road is proposed to provide a shared through/right turn lane, while the westbound approach is proposed to provide a shared left turn/through lane. The northbound approach of the site driveway is proposed to provide a shared left/right turn lane.

As designed, the driveway is anticipated to operate at levels of service "B" or better during the studied peak hours. See Table V for the individual movement levels of service and delays.



### **SITE PLAN**

### Site Access and Circulation

The site plan was reviewed with respect to the site access and on-site circulation design. As noted previously, access to The Project will be provided via a new full movement driveway along Vanderburg Road.

The parking lot will be serviced by parking aisles with minimum widths of 30' for two-way circulation, which satisfies the Ordinance's minimum requirement of 25'. Review of the site plan design indicates that the site can sufficiently accommodate, within paved areas, a large wheel base vehicle, such as a fire truck or recreational vehicle, along with the automobile traffic anticipated.

### **Parking**

The Marlboro Township Ordinance sets forth a parking requirement of 1 parking space per 5,000 SF for warehouse, storage, distribution, shipping and receiving facilities. This equates to a parking requirement of 18 spaces for the proposed 89,700 SF self-storage facility. The site as proposed provides 67 parking spaces, and as such the Ordinance requirement is satisfied.

It is proposed to provide parking stalls with dimensions of 10'x20, 12'x20' and 12'x30', which satisfy the Ordinance minimum requirement of 10'x20'.



### FINDINGS AND CONCLUSIONS

### **Findings**

Based upon the detailed analyses as documented herein, the following findings are noted:

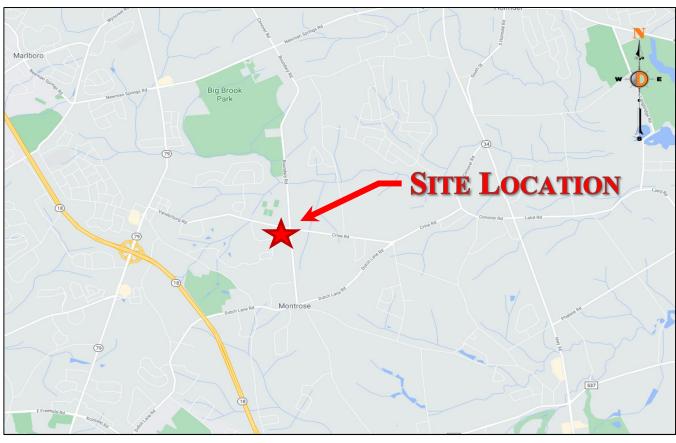
- The proposed 89,700 SF Self-Storage Facility, is projected to generate 5 entering trips and 4 exiting trips during the weekday morning peak hour, 7 entering trips and 8 exiting trips during the evening peak hour, and 17 entering trips and 11 exiting trips during the Saturday peak hour that are "new" to the adjacent roadway network.
- Access to the site is proposed to be provided via a new full movement driveway along Vanderburg Road.
- With the addition of site generated traffic, the intersection of Crine Road (CR 4)/Vanderburg Road and Boundary Road is anticipated to operate at No Build levels of service "B" or better during the peak hours studied.
- As designed, the intersection of Vanderburg Road and site driveway is anticipated to operate at levels of service "B" or better during the peak hours studied.
- As proposed, The Project's site driveways and internal circulation have been designed to provide for safe and efficient movement of automobiles and large wheel base vehicles.
- The proposed parking supply and design is sufficient to support the projected demand and satisfies the Ordinance requirements.

### **Conclusions**

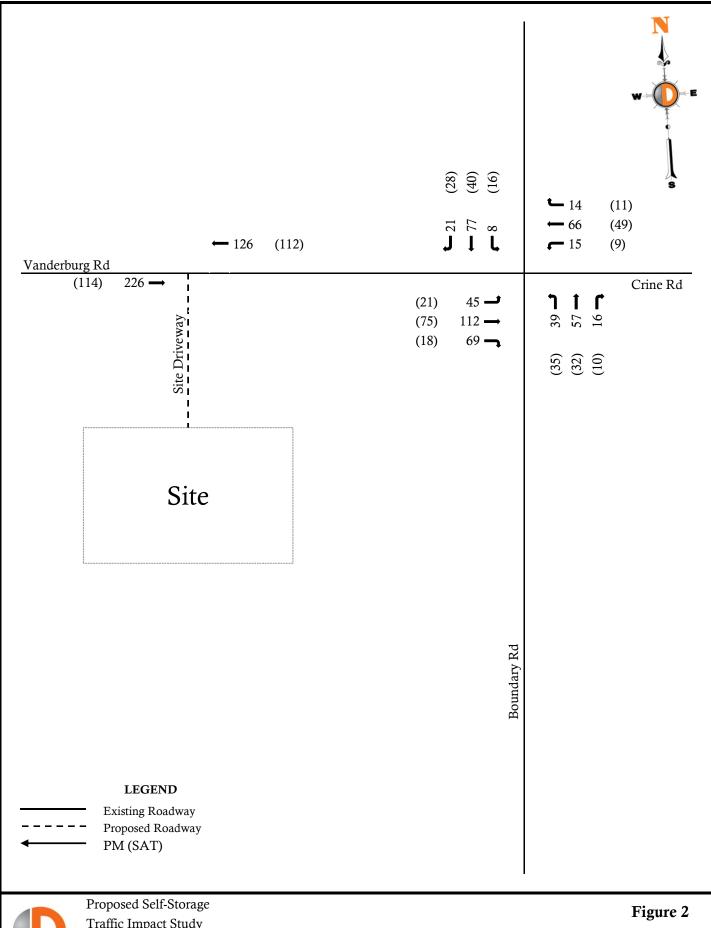
Based upon our Traffic Impact Study as detailed in the body of this report, it is the professional opinion of Dynamic Traffic LLC that the adjacent street system of Marlboro Township, Colts Neck Township and Monmouth County will not experience any significant degradation in operating conditions with the construction of The Project. The site driveway is located to provide safe and efficient access to the adjacent roadway system. The site plan as proposed provides for good circulation throughout the site and provides adequate parking to accommodate The Project's needs.

Appendix A Traffic Volume Figures

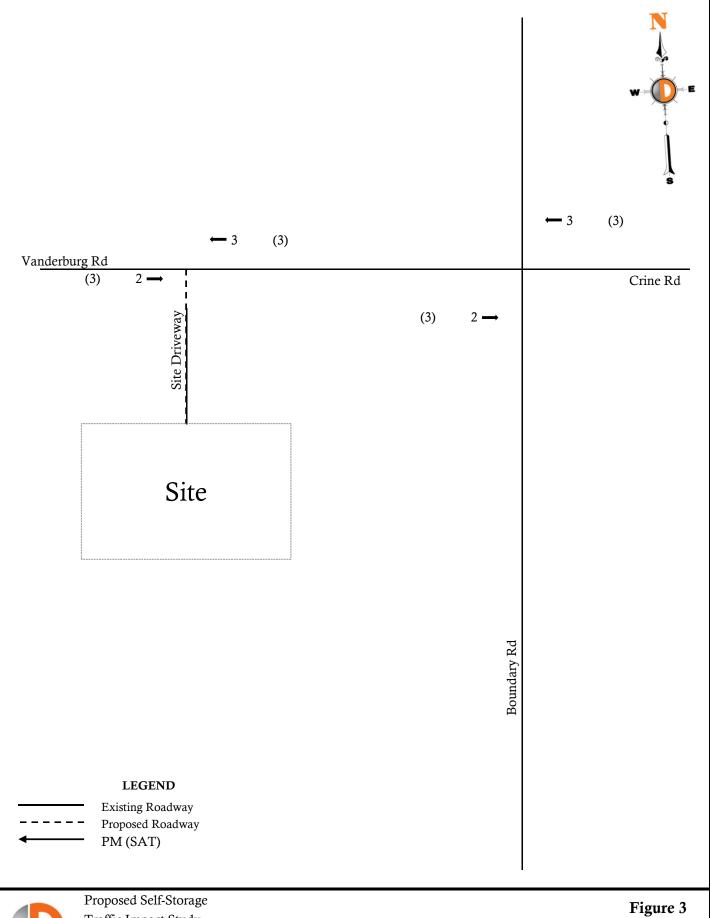






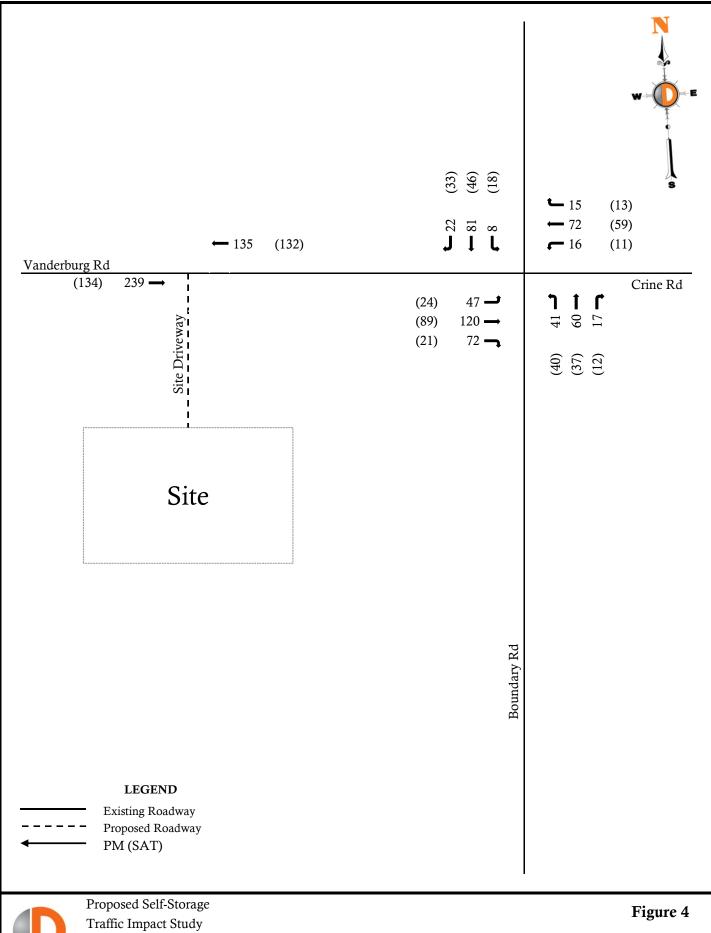






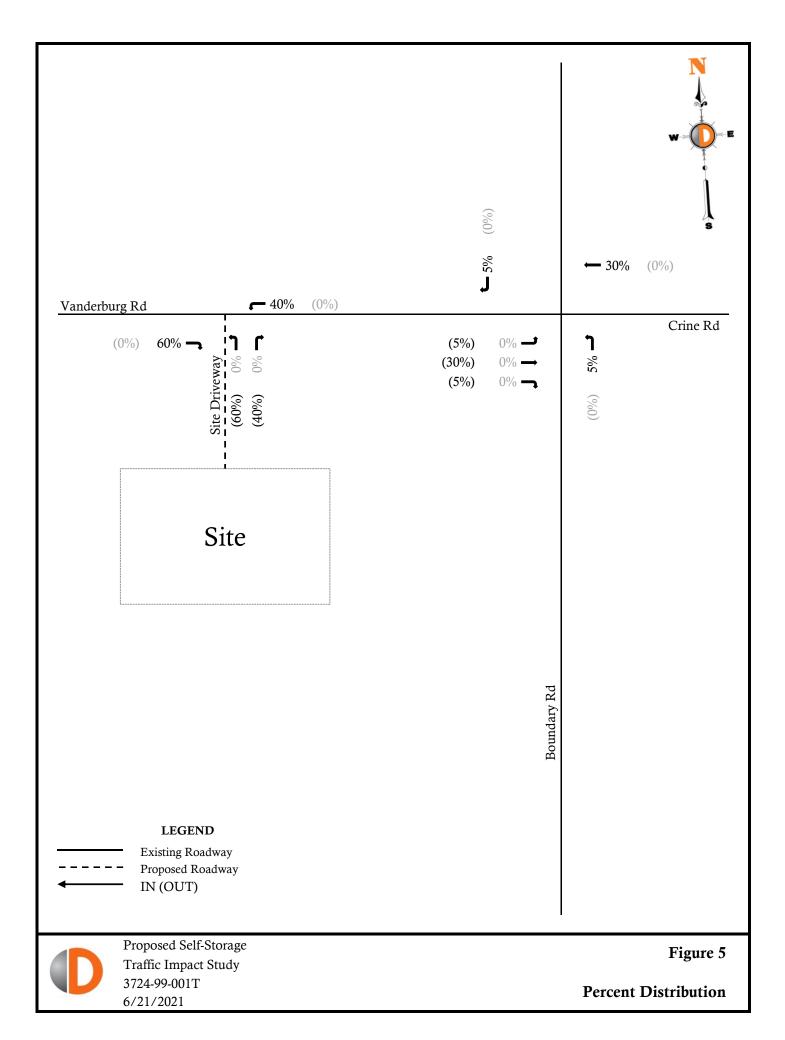


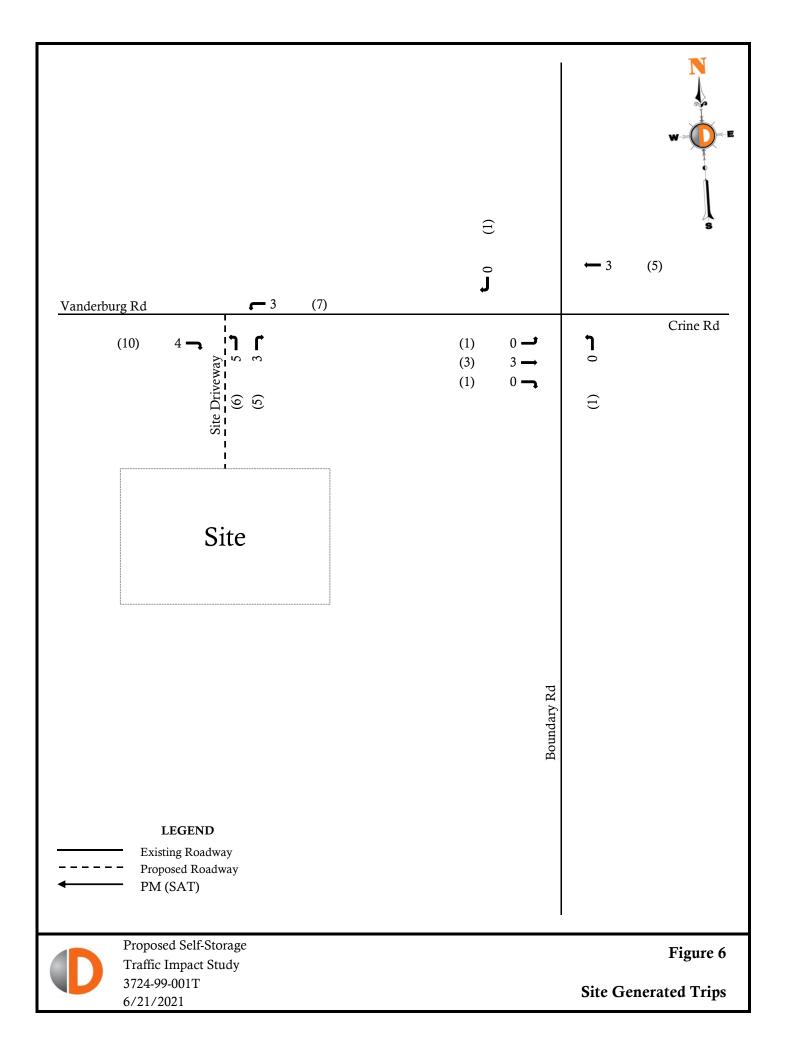
Adjacent Development Traffic Volumes
[Marlboro Green]

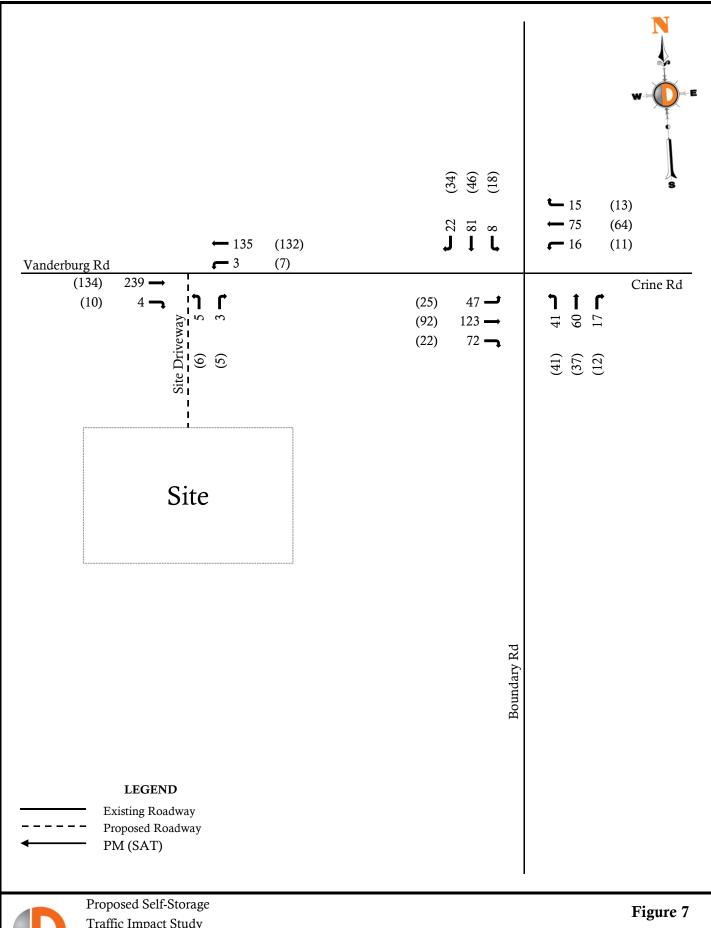




3724-99-001T 6/21/2021







D

Appendix B Project Information

# Dynamic Traffic, LLC

1904 Main Street, Lake Como, NJ 07719 245 Main Street - Suite 110, Chester, NJ 07930 732-681-0760

E/W: Vanderburg Rd/Crine Rd File Name: Vanderburg Rd-Crine Rd & Boundary Rd - PM

N/S: Boundary Rd Site Code : 00000000 Town/County: Marlboro/Monmouth Start Date : 6/10/2021

Job #: 3724-99-001T Page No : 1

Groups Printed- Cars - Trucks (SU) - Trucks (TT)

		Vand	erbur	g Roa	d		Cı	rine R	oad		LICITO	·	ndary	Road	, I		Bou	ndary	Road		
		E	astbo	und			W	estbo	und				rthbo				So	uthbo	ound		
Start Time	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Int. Total
04:30 PM	12	30	21	0	63	2	15	4	0	21	6	9	5	0	20	2	21	5	0	28	132
04:45 PM	11	17	14	0	42	2	18	4	0	24	13	12	3	0	28	1	24	6	0	31	125
Total	23	47	35	0	105	4	33	8	0	45	19	21	8	0	48	3	45	11	0	59	257
	1										ı					I					ı
05:00 PM	12	43	22	0	77	7	19	4	0	30	8	11	2	0	21	2	20	2	0	24	152
05:15 PM	10	22	12	0	44	4	14	2	0	20	12	25	6	0	43	3	12	8	0	23	130
05:30 PM	10	29	16	0	55	2	16	3	0	21	11	14	3	0	28	4	16	0	0	20	124
05:45 PM	7	21	12	0	40	4	12	5	0	21	20	11	0	0	31	3	14	4	0	21	113
Total	39	115	62	0	216	17	61	14	0	92	51	61	11	0	123	12	62	14	0	88	519
				_												l .					
06:00 PM	5	18	11	0	34	3	13	1	0	17	9	10	3	0	22	4	10	10	0	24	97
06:15 PM	8	20	7	0	35	3	18	2	0	23	9	11	4	0	24	3	14	10	0	27	109
Grand Total	75	200	115	0	390	27	125	25	0	177	88	103	26	0	217	22	131	45	0	198	982
Apprch %		51.3	29.5	0		15.3	70.6	14.1	0		40.6	47.5	12	0		11.1	66.2	22.7	0		
Total %	7.6	20.4	11.7	0	39.7	2.7	12.7	2.5	0	18	9	10.5	2.6	0	22.1	2.2	13.3	4.6	0	20.2	
Cars	74	198	111	0	383	27	124	25	0	176	84	103	26	0	213	21	129	45	0	195	967
% Cars	98.7	99	96.5	0	98.2	100	99.2	100	0	99.4	95.5	100	100	0	98.2	95.5	98.5	100	0	98.5	98.5
Trucks (SU)	1	1	1	0	3	0	0	0	0	0	2	0	0	0	2	1	2	0	0	3	8
% Trucks (SU)	1.3	0.5	0.9	0	0.8	0	0	0	0	0	2.3	0	0	0	0.9	4.5	1.5	0	0	1.5	0.8
Trucks (TT)	0	1	3	0	4	0	1	0	0	1	2	0	0	0	2	0	0	0	0	0	7
% Trucks (TT)	0	0.5	2.6	0	1	0	8.0	0	0	0.6	2.3	0	0	0	0.9	0	0	0	0	0	0.7

			erburg astbo	g Road und	b		_	ine R					ndary orthbo	Road				ndary	Road		
Start Time	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Int. Total
Peak Hour	Analys	is Fro	m 04:	30 PM	to 06:1	5 PM	- Peal	k 1 of	1												
Peak Hour t	or Ent	ire Int	ersect	ion Be	gins at	04:30	PM														ı
04:30 PM	12	30	21	0	63	2	15	4	0	21	6	9	5	0	20	2	21	5	0	28	132
04:45 PM	11	17	14	0	42	2	18	4	0	24	13	12	3	0	28	1	24	6	0	31	125
05:00 PM	12	43	22	0	77	7	19	4	0	30	8	11	2	0	21	2	20	2	0	24	152
_05:15 PM	10	22	12	0	44	4	14	2	0	20	12	25	6	0	43	3	12	8	0	23	130
Total Volume	45	112	69	0	226	15	66	14	0	95	39	57	16	0	112	8	77	21	0	106	539
% App. Total	19.9	49.6	30.5	0		15.8	69.5	14.7	0		34.8	50.9	14.3	0		7.5	72.6	19.8	0		
PHF	.938	.651	.784	.000	.734	.536	.868	.875	.000	.792	.750	.570	.667	.000	.651	.667	.802	.656	.000	.855	.887
Cars	45	112	66	0	223	15	65	14	0	94	36	57	16	0	109	7	77	21	0	105	531
% Cars	100	100	95.7	0	98.7	100	98.5	100	0	98.9	92.3	100	100	0	97.3	87.5	100	100	0	99.1	98.5
Trucks (SU)	0	0	1	0	1	0	0	0	0	0	1	0	0	0	1	1	0	0	0	1	3
% Trucks (SU)	0	0	1.4	0	0.4	0	0	0	0	0	2.6	0	0	0	0.9	12.5	0	0	0	0.9	0.6
Trucks (TT)	0	0	2	0	2	0	1	0	0	1	2	0	0	0	2	0	0	0	0	0	5
% Trucks (TT)	0	0	2.9	0	0.9	0	1.5	0	0	1.1	5.1	0	0	0	1.8	0	0	0	0	0	0.9

# Dynamic Traffic, LLC

1904 Main Street, Lake Como, NJ 07719 245 Main Street - Suite 110, Chester, NJ 07930 732-681-0760

E/W: Vanderburg Rd/Crine Rd File Name: Vanderburg Rd-Crine Rd & Boundary Rd - SAT

N/S: Boundary Rd Site Code : 00000000 Town/County: Marlboro/Monmouth Start Date : 6/12/2021

Job #: 3724-99-001T Page No : 1

Groups Printed- Cars - Trucks (SU) - Trucks (TT)

				g Roa	d			ine R						Road	ĺ				Road		
			astbo	und				estbo	und			No	rthbo					uthbo			
Start Time	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Int. Total
11:00 AM	4	13	5	0	22	5	22	4	0	31	4	7	5	0	16	1	10	8	0	19	88
11:15 AM	9	13	3	0	25	3	22	0	0	25	4	10	3	0	17	0	7	6	0	13	80
11:30 AM	5	15	5	0	25	4	15	4	0	23	6	5	3	0	14	3	6	4	0	13	75
11:45 AM	2	13	6	0	21	5	18	8	0	31	6	10	0	0	16	3	10	7	0	20	88
Total	20	54	19	0	93	17	77	16	0	110	20	32	11	0	63	7	33	25	0	65	331
						ı					I				1						ı
12:00 PM	5	23	6	0	34	3	11	3	0	17	7	7	2	0	16	3	8	6	0	17	84
12:15 PM	2	11	3	0	16	3	19	3	0	25	6	13	6	0	25	1	12	2	0	15	81
12:30 PM	3	19	3	0	25	2	12	0	0	14	12	8	2	0	22	2	12	4	0	18	79
_12:45 PM	5	17	2	0	24	4	13	1_	0	18	9	7	2	0	18	2	9	6	0	17	77
Total	15	70	14	0	99	12	55	7	0	74	34	35	12	0	81	8	41	18	0	67	321
											ı										ı
01:00 PM	6	21	10	0	37	1	11	5	0	17	5	10	2	0	17	4	10	7	0	21	92
01:15 PM	6	21	3	0	30	1	17	2	0	20	8	7	4	0	19	5	8	8	0	21	90
01:30 PM	4	16	3	0	23	3	8	3	0	14	13	8	2	0	23	5	13	7	0	25	85
_01:45 PM	4	15_	13	0	32	3	9	0	0	12	4	7	1	0	12	3	5	1	0	9	65
Total	20	73	29	0	122	8	45	10	0	63	30	32	9	0	71	17	36	23	0	76	332
				_							1				1						
Grand Total	55	197	62	0	314	37	177	33	0	247	84	99	32	0	215	32	110	66	0	208	984
Apprch %	17.5	62.7	19.7	0		15	71.7	13.4	0		39.1	46	14.9	0		15.4	52.9	31.7	0		
Total %	5.6	20	6.3	0	31.9	3.8	18	3.4	0	25.1	8.5	10.1	3.3	0	21.8	3.3	11.2	6.7	0	21.1	
Cars	54	197	62	0	313	36	176	32	0	244	83	95	31	0	209	32	109	65	0	206	972
% Cars	98.2	100	100	0	99.7	97.3	99.4	97	0_	98.8	98.8	96	96.9	0	97.2	100	99.1	98.5	0_	99	98.8
Trucks (SU)	1	0	0	0	1	1	1	1	0	3	1	4	1	0	6	0	1	1	0	2	12
% Trucks (SU)	1.8	0	0	0	0.3	2.7	0.6	3	0	1.2	1.2	4	3.1	0	2.8	0	0.9	1.5	0	1	1.2
Trucks (TT)	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
% Trucks (TT)	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

			erburg astbo	_	d			rine R					ndary	Road				ndary	Road	I	
		1	astbot			Westbound						טמוווווכ					uthbo				
Start Time	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Int. Total
Peak Hour /	Analys	sis Fro	m 11:0	MA 00	to 01:4	5 PM	- Peal	k 1 of	1												
Peak Hour f	or Ent	tire Int	ersect	ion Be	gins at	12:45	PM														
12:45 PM	5	17	2	0	24	4	13	1	0	18	9	7	2	0	18	2	9	6	0	17	77
01:00 PM	6	21	10	0	37	1	11	5	0	17	5	10	2	0	17	4	10	7	0	21	92
01:15 PM	6	21	3	0	30	1	17	2	0	20	8	7	4	0	19	5	8	8	0	21	90
01:30 PM	4	16	3	0	23	3	8	3	0	14	13	8	2	0	23	5	13	7	0	25	85
Total Volume	21	75	18	0	114	9	49	11	0	69	35	32	10	0	77	16	40	28	0	84	344
% App. Total	18.4	65.8	15.8	0		13	71	15.9	0		45.5	41.6	13	0		19	47.6	33.3	0		
PHF	.875	.893	.450	.000	.770	.563	.721	.550	.000	.863	.673	.800	.625	.000	.837	.800	.769	.875	.000	.840	.935
Cars	21	75	18	0	114	9	48	11	0	68	34	31	10	0	75	16	40	28	0	84	341
% Cars	100	100	100	0	100	100	98.0	100	0	98.6	97.1	96.9	100	0	97.4	100	100	100	0	100	99.1
Trucks (SU)	0	0	0	0	0	0	1	0	0	1	1	1	0	0	2	0	0	0	0	0	3
% Trucks (SU)	0	0	0	0	0	0	2.0	0	0	1.4	2.9	3.1	0	0	2.6	0	0	0	0	0	0.9
Trucks (TT)	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
% Trucks (TT)	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1 (

# Dynamic Traffic, LLC

1904 Main Street, Lake Como, NJ 07719 245 Main Street - Suite 110, Chester, NJ 07930 732-681-0760

E/W: Amboy Rd File Name: Tennent Rd & Amboy Rd - AMPM

N/S: Tennent Rd Site Code : 00000000 Town/County: Marlboro/Monmuoth Start Date : 4/20/2021

Job #: 3342-99-003T Page No : 1

Groups	Printed- Cars - Trucks	<u>(SU</u>	) - Trucks (	(TT)	)_

			nboy R					nnent R					nnent R			
		E	astbou	nd			N	orthbou				S	outhbo	und		
Start Time	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Int. Total
07:00 AM	3	0	17	0	20	19	64	0	0	83	0	63	1	0	64	167
07:15 AM	4	0	15	1	20	24	89	0	0	113	0	87	3	0	90	223
07:30 AM	3	0	21	0	24	18	88	0	0	106	0	103	2	0	105	235
07:45 AM	2	0	21	0	23	25	76	0	0	101	0	109	4	0	113	237
Total	12	0	74	1	87	86	317	0	0	403	0	362	10	0	372	862
00.00 444	•	0	20	0	45	24	0.4	0	0	440		101	•	0	407	070
08:00 AM	6	0	39	0	45	24	94	0	0	118	0	101	6	0	107	270
08:15 AM	3	0	43	0	46	38	108	0	0	146	0	122	4	0	126	318
08:30 AM	6	0	42	0	48	53	111	0	0	164	0	95	6	0	101	313
08:45 AM	9	0	51	0	60	34	101	0	0	135	0	111	14	0	125	320
Total	24	0	175	0	199	149	414	0	0	563	0	429	30	0	459	1221
*** BREAK ***																
04:30 PM	10	0	33	0	43	31	119	0	0	150	0	118	4	0	122	315
04:45 PM	7	0	37	0	44	51	122	0	0	173	0	122	14	0	136	353
Total	17	0	70	0	87	82	241	0	0	323	0	240	18	0	258	668
05.00.514	-	•	50	•		40	400	•	•	470	١ ٥	440	•	•	400	0.57
05:00 PM	7	0	50	0	57	42	136	0	0	178	0	119	3	0	122	357
05:15 PM	11	0	47	0	58	49	168	0	0	217	0	126	12	0	138	413
05:30 PM	11	0	49	0	60	50	161	0	0	211	0	126	15	0	141	412
05:45 PM	13	0	32	0	45	45	157	0	0	202	0	91	16	0	107	354
Total	42	0	178	0	220	186	622	0	0	808	0	462	46	0	508	1536
06:00 PM	3	0	55	0	58	33	144	0	0	177	0	134	8	0	142	377
06:15 PM	3	0	54	0	57	18	75	0	0	93	0	104	6	0	110	260
Grand Total	101	0	606	1	708	554	1813	0	0	2367	0	1731	118	0	1849	4924
Apprch %	14.3	0	85.6	0.1		23.4	76.6	0	0		0	93.6	6.4	0		
Total %	2.1	0	12.3	0.1	14.4	11.3	36.8	0	0	48.1	0	35.2	2.4	0	37.6	
Cars	94	0	573	1	668	499	1759	0	0	2258	0	1674	112	0	1786	4712
% Cars	93.1	0	94.6	100	94.4	90.1	97	0	0	95.4	Ö	96.7	94.9	0	96.6	95.7
Trucks (SU)	7	0	32	0	39	52	48	0	0	100	0	54	6	0	60	199
% Trucks (SU)	6.9	0	5.3	0	5.5	9.4	2.6	0	0	4.2	0	3.1	5.1	0	3.2	4
Trucks (TT)	0.0	0	1	0	1	3	6	0	0	9	0	3	0.1	0	3	13
% Trucks (TT)	0	0	0.2	0	0.1	0.5	0.3	Ö	0	0.4	Ö	0.2	0	0	0.2	0.3
( )	•	-		•				-	•		,		-	•		

# Dynamic Traffic, LLC 1904 Main Street, Lake Como, NJ 07719

1904 Main Street, Lake Como, NJ 07719 245 Main Street - Suite 110, Chester, NJ 07930 732-681-0760

E/W: Amboy Rd File Name: Tennent Rd & Amboy Rd - AMPM

N/S: Tennent Rd Site Code : 00000000 Town/County: Marlboro/Monmuoth Start Date : 4/20/2021

Job #: 3342-99-003T Page No : 2

			nboy R astbou					nnent R					nnent R			
Start Time	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Int. Total
Peak Hour Ana				11:45 A		1 of 1	•				•					
Peak Hour for E	ntire Int	ersectio	n Begir	ns at 08:	:00 AM											
08:00 AM	6	0	39	0	45	24	94	0	0	118	0	101	6	0	107	270
08:15 AM	3	0	43	0	46	38	108	0	0	146	0	122	4	0	126	318
08:30 AM	6	0	42	0	48	53	111	0	0	164	0	95	6	0	101	313
08:45 AM	9	0	51	0	60	34	101	0	0	135	0	111	14	0	125	320
Total Volume	24	0	175	0	199	149	414	0	0	563	0	429	30	0	459	1221
% App. Total	12.1	0	87.9	0		26.5	73.5	0	0		0	93.5	6.5	0		
PHF	.667	.000	.858	.000	.829	.703	.932	.000	.000	.858	.000	.879	.536	.000	.911	.954
Cars	21	0	153	0	174	117	397	0	0	514	0	407	27	0	434	1122
% Cars	87.5	0	87.4	0	87.4	78.5	95.9	0	0	91.3	0	94.9	90.0	0	94.6	91.9
Trucks (SU)	3	0	21	0	24	32	17	0	0	49	0	22	3	0	25	98
% Trucks (SU)	12.5	0	12.0	0	12.1	21.5	4.1	0	0	8.7	0	5.1	10.0	0	5.4	8.0
Trucks (TT)	0	0	1	0	1	0	0	0	0	0	0	0	0	0	0	1
% Trucks (TT)	0	0	0.6	0	0.5	0	0	0	0	0	0	0	0	0	0	0.1
Peak Hour Ana Peak Hour for E						1 of 1										
05:15 PM	11	0	47	0	58	49	168	0	0	217	0	126	12	0	138	413
05:30 PM	11	0	49	0	60	50	161	0	0	211	0	126	15	0	141	412
05:45 PM	13	0	32	0	45	45	157	0	0	202	0	91	16	0	107	354
06:00 PM	3	0	55	0	58	33	144	0	0	177	0	134	8	0	142	377
Total Volume	38	0	183	0	221	177	630	0	0	807	0	477	51	0	528	1556
% App. Total	17.2	0	82.8	0		21.9	78.1	0	0		0	90.3	9.7	0		
PHF	.731	.000	.832	.000	.921	.885	.938	.000	.000	.930	.000	.890	.797	.000	.930	.942
Cars	37	0	182	0	219	174	626	0	0	800	0	474	49	0	523	1542
% Cars	97.4	0	99.5	0	99.1	98.3	99.4	0	0	99.1	0	99.4	96.1	0	99.1	99.1
Trucks (SU)	1	0	1	0	2	2	2	0	0	4	0	3	2	0	5	11
% Trucks (SU)	2.6	0	0.5	0	0.9	1.1	0.3	0	0	0.5	0	0.6	3.9	0	0.9	0.7
Trucks (TT)	0	0	0	0	0	1	2	0	0	3	0	0	0	0	0	3
% Trucks (TT)	0	0	0	0	0	0.6	0.3	0	0	0.4	0	0	0	0	0	0.2

# New Jersey Department of Transportation

Short-term Hourly Traffic Volume for 06/25/2018 to 06/28/2018

121304, Tennent Road-6.79, 13000003

MONMOUTH Site names: County: Funct Class: Location:

Urban Minor Arterial BET AMBOY RD CRINE RD

Seasonal Factor Grp: Daily Factor Grp:

rg3\_4U rg3\_4U rg3\_4U rg3\_4U Axle Factor Grp: Growth Factor Grp:

2018	တ																											
Sat, Jun 30,	z																											
Sa	Road																											
2018	S																											
Fri, Jun 29, 2018	z																											
Fr	Road																											
2018	S	53	19	26	6	4	18	49	135	307																620		
Thu, Jun 28, 2	z	74	21	30	15	22	16	31	95	201																488		
Thu,	Road	127	40	99	24	o	34	80	230	208																1,108		
2018	S	21	46	20	10	8	27	92	264	439	551	446	368	391	388	374	426	429	478	476	397	370	280	200	142	6,643	551	912
Wed, Jun 27, 20	z	30	39	24	11	∞	32	64	174	280	401	322	309	363	385	385	414	415	483	581	464	387	307	254	122	6,254	401	116.
Wed,	Road	51	85	44	21	16	29	156	438	719	952	292	229	754	773	759	840	844	961	1,057	861	157	287	454	264	12,897	952	786.
2018	S	13	16	9	10	15	52	151	329	528	202	384	383	418	416	420	402	451	460	468	447	344	223	151	92	869'9	573	.924
Jun 26, 2	z	16	19	14	10	21	32	66	273	368	387	346	353	385	411	401	415	464	541	220	429	335	284	235	110	6,501	398	906
Tue,	Road	53	32	20	20	36	87	250	632	968	892	730	736	803	827	821	817	915	1,001	1,018	876	629	202	386	186	13,199	971	26.
018	S													426	417	362	403	446	513	423	349	221	206	141	65	3,972		
Mon, Jun 25, 2018	z													386	391	384	399	468	518	479	425	291	249	145	82	4,217		
Mon,	Road													812	808	746	802	914	1,031	905	774	512	455	286	147	8,189		
2018	S																											
Sun, Jun 24, 2018	z																											
Sun,	Road																											
		00:00	01:00	02:00	03:00	04:00	02:00	00:90	07:00	08:00	00:60	10:00	11:00	12:00	13:00	14:00	15:00	16:00	17:00	18:00	19:00	20:00	21:00	22:00	23:00	Total	AM Peak Vol	AM Peak Fct

..941 ..928 ..486 2.000

.941 .928 .486 2.000

.941 .928 .486 2.000

.941 .945 .486 2.000

.945 .486 2.000

.941 .945 .486 2.000

.981 .486 2.000

.941 .981 .486 2.000

.941 .981 .486 2.000

.941 1.004 .486 2.000

.941 1.004 .486 2.000

.941 1.004 .486 2.000

9: 00 505 .908 .7: 15

9:00

9: 00 1,073 .897 17: 30

8: 30 504 .962 16: 30

8: 30 550 .975 .18: 00

8: 30 1,036 .949 16: 30

.75 17: 00 513

.887 17: 30 532

1,031 .889 17: 00

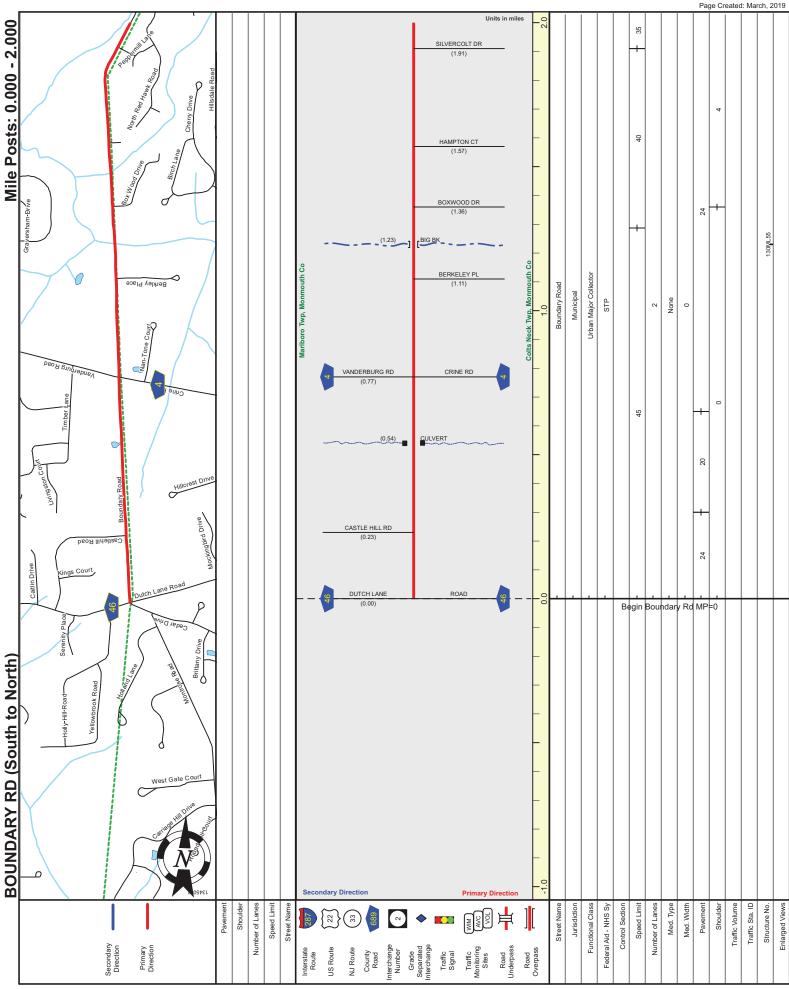
PM Peak Fct PM Peak Hr

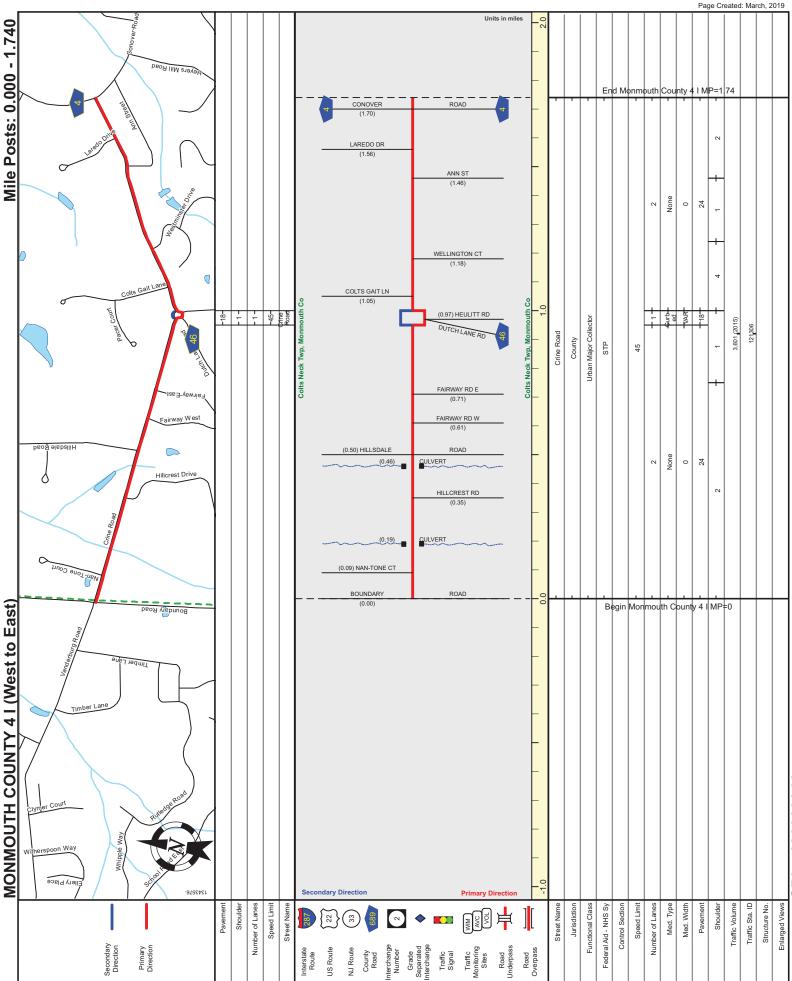
Seasonal Fct

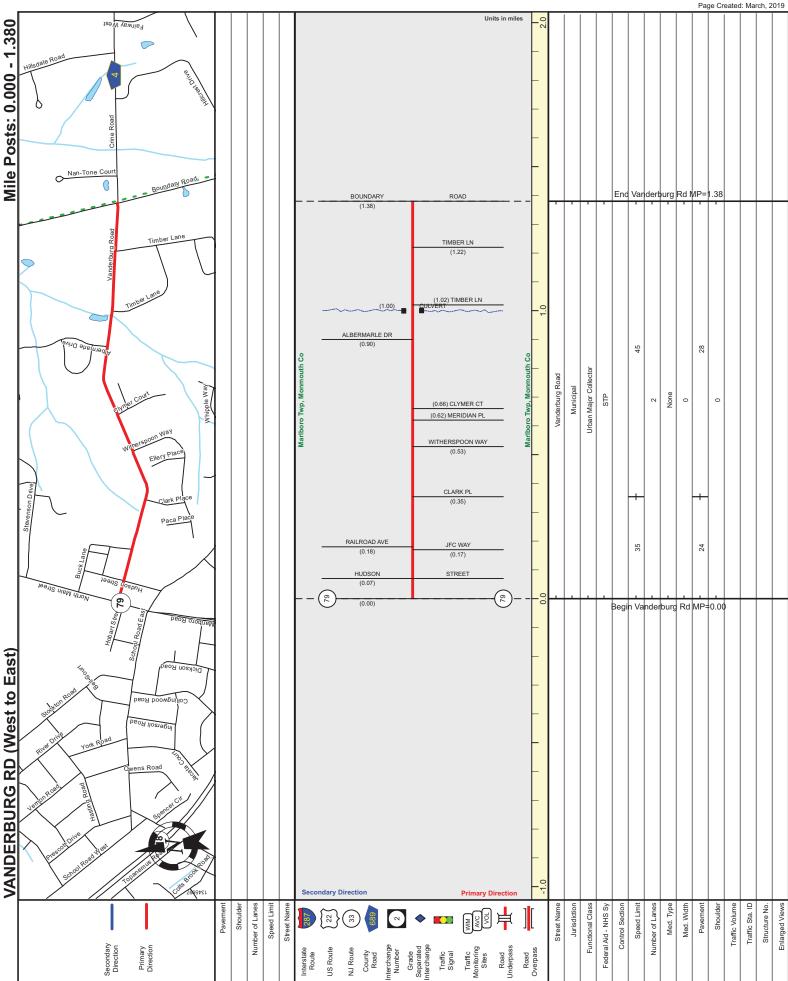
Daily Fct Axle Fct Pulse Fct

PM Peak Vol

AM Peak Hr







Appendix C Capacity Analysis

Intersection												
Int Delay, s/veh	6.7											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	45	112	69	15	66	14	39	57	16	8	77	21
Future Vol, veh/h	45	112	69	15	66	14	39	57	16	8	77	21
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	-2	-	-	4	-	-	-2	-	-	6	-
Peak Hour Factor	89	89	89	89	89	89	89	89	89	89	89	89
Heavy Vehicles, %	0	0	3	0	2	0	8	0	0	13	0	0
Mvmt Flow	51	126	78	17	74	16	44	64	18	9	87	24
Major/Minor N	Major1		<u> </u>	Major2			Minor1		<u> </u>	Minor2		
Conflicting Flow All	90	0	0	204	0	0	439	391	165	424	422	82
Stage 1	-	-	-	-	-	-	267	267	-	116	116	-
Stage 2	-	-	-	-	-	-	172	124	-	308	306	-
Critical Hdwy	4.1	-	-	4.1	-	-	6.78	6.1	6	8.43	7.7	6.8
Critical Hdwy Stg 1	-	-	-	-	-	-	5.78	5.1	-	7.43	6.7	-
Critical Hdwy Stg 2	-	-	-	-	-	-	5.78	5.1	-	7.43	6.7	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.572	4	3.3	3.617	4	3.3
Pot Cap-1 Maneuver	1518	-	-	1380	-	-	544	572	893	453	457	970
Stage 1	-	-	-	-	-	-	747	713	-	830	773	-
Stage 2	-	-	-	-	-	-	832	808	-	613	601	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1518	-	-	1380	-	-	432	543	893	389	434	970
Mov Cap-2 Maneuver	-	-	-	-	-	-	432	543	-	389	434	-
Stage 1	-	-	-	-	-	-	719	686	-	798	763	-
Stage 2	-	-	-	-	-	-	710	797	-	524	578	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	1.5			1.2			14			14.9		
HCM LOS							В			В		
										_		
Minor Lane/Major Mvm	t N	IBLn1	EBL	EBT	EBR	WBL	WBT	WBR :	SBI n1			
Capacity (veh/h)		525	1518			1380	-	-				
HCM Lane V/C Ratio			0.033	_		0.012	_		0.247			
HCM Control Delay (s)		14	7.5	0		7.6	0	_	14.9			
HCM Lane LOS		В	7.5 A	A	_	Α.	A	_	В			
HCM 95th %tile Q(veh)		0.9	0.1	-	_	0	-	_	1			
		0.0	J. 1									

Intersection												
Int Delay, s/veh	5.9											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	23	82	20	10	53	12	38	35	11	17	44	31
Future Vol, veh/h	23	82	20	10	53	12	38	35	11	17	44	31
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	-2	-	-	4	-	-	-2	-	-	6	-
Peak Hour Factor	94	94	94	94	94	94	94	94	94	94	94	94
Heavy Vehicles, %	0	0	0	0	2	0	3	3	0	0	0	0
Mvmt Flow	24	87	21	11	56	13	40	37	12	18	47	33
Major/Minor N	Major1		1	Major2			Minor1		N	/linor2		
Conflicting Flow All	69	0	0	108	0	0	271	237	98	255	241	63
Stage 1	-	-	-	-	-	-	146	146	-	85	85	-
Stage 2	_	_	_	_	_	_	125	91	_	170	156	_
Critical Hdwy	4.1	-	-	4.1	_	-	6.73	6.13	6	8.3	7.7	6.8
Critical Hdwy Stg 1	-	-	_		_	-	5.73	5.13	-	7.3	6.7	-
Critical Hdwy Stg 2	-	-	_	-	_	-	5.73	5.13	-	7.3	6.7	-
Follow-up Hdwy	2.2	-	_	2.2	_	-	3.527	4.027	3.3	3.5	4	3.3
Pot Cap-1 Maneuver	1545	-	-	1495	_	-	700	680	969	645	613	997
Stage 1	-	-	_	-	_	-	868	787	-	902	805	-
Stage 2	-	_	_	-	-	-	889	826	-	791	733	-
Platoon blocked, %		-	_		_	-						
Mov Cap-1 Maneuver	1545	-	_	1495	_	-	624	663	969	599	598	997
Mov Cap-2 Maneuver	-	-	-	-	-	-	624	663	-	599	598	-
Stage 1	-	-	_	-	-	-	853	774	-	887	799	-
Stage 2	-	-	_	_	_	-	803	819	-	731	721	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	1.4			1			11.2			11.1		
HCM LOS	1.7						11.2 B			В		
TOW LOO							U			U		
Minor Long/Major Mare	4	NBLn1	EDI	EDT	EBR	WDI	WDT	WDD	CDI ~1			
Minor Lane/Major Mvm	l l		EBL	EBT		WBL	WBT	WBR				
Capacity (veh/h)		672		-	-	1495	-	-	691			
HCM Control Doloy (a)		0.133		-		0.007	-		0.142			
HCM Control Delay (s)		11.2	7.4	0	-	7.4	0	-	11.1			
HCM CEth (/tile O/ceh)		В	A	Α	-	A	Α	-	В			
HCM 95th %tile Q(veh)		0.5	0	-	-	0	-	-	0.5			

Intersection												
Int Delay, s/veh	6.9											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4	LDIX	WDL .	4	WER	IIDL	4	HOIL	ODL	4	ODIT
Traffic Vol, veh/h	47	120	72	16	72	15	41	60	17	8	81	22
Future Vol, veh/h	47	120	72	16	72	15	41	60	17	8	81	22
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	- -	-	None
Storage Length	_	_	-	_	_	-	_	_	-	_	_	-
Veh in Median Storage	.# -	0	_	_	0	_	_	0	_	_	0	_
Grade, %	, _	-2	_	_	4	_	_	-2	_	_	6	_
Peak Hour Factor	89	89	89	89	89	89	89	89	89	89	89	89
Heavy Vehicles, %	0	0	3	0	2	0	8	0	0	13	0	0
Mvmt Flow	53	135	81	18	81	17	46	67	19	9	91	25
Major/Minor N	Major1		ı	Major2			Minor1		ı	Minor2		
Conflicting Flow All	98	0	0	216	0	0	466	416	176	451	448	90
Stage 1	90	-	U	210	-	-	282	282	170	126	126	90
Stage 2	_	-		-	_	-	184	134	-	325	322	-
Critical Hdwy	4.1	-	-	4.1	-	-	6.78	6.1	6	8.43	7.7	6.8
Critical Hdwy Stg 1	7.1		_	4.1			5.78	5.1	_	7.43	6.7	J.U _
Critical Hdwy Stg 2		-	_	_		_	5.78	5.1	_	7.43	6.7	_
Follow-up Hdwy	2.2	_	_	2.2	_	_	3.572	4		3.617	4	3.3
Pot Cap-1 Maneuver	1508	_	_	1366	_	_	523	555	881	431	438	959
Stage 1	-	_	-	-	-	_	735	703	-	817	763	-
Stage 2	_	_	_	_	_	_	821	801	_	597	588	_
Platoon blocked, %		_	_		_	_	J <u>L</u> ,	- 501		301	300	
Mov Cap-1 Maneuver	1508	-	-	1366	-	-	407	526	881	365	415	959
Mov Cap-2 Maneuver	-	_	_	-	_	-	407	526	-	365	415	
Stage 1	_	_	_	_	_	-	706	675	_	784	752	-
Stage 2	-	-	-	-	-	-	693	790	-	505	564	-
<b>J</b> -												
Approach	EB			WB			NB			SB		
HCM Control Delay, s	1.5			1.2			14.7			15.6		
HCM LOS	1.5						В			C		
Minor Lane/Major Mvm	t	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR :	SBI n1			
Capacity (veh/h)		504	1508		-	1366		.,5,,,	462			
HCM Lane V/C Ratio		0.263	0.035	_		0.013	_	_	0.27			
HCM Control Delay (s)		14.7	7.5	0	_	7.7	0	_	15.6			
HCM Lane LOS		В	7.5 A	A	_	Α	A	_	C			
HCM 95th %tile Q(veh)		1	0.1	-		0	-	_	1.1			
HOW JOHN JUNIO Q(VOII)		1	J. 1			U			1.1			

Intersection												
Int Delay, s/veh	5.9											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	24	89	21	11	59	13	40	37	12	18	46	33
Future Vol, veh/h	24	89	21	11	59	13	40	37	12	18	46	33
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	-2	-	-	4	-	-	-2	-	-	6	-
Peak Hour Factor	94	94	94	94	94	94	94	94	94	94	94	94
Heavy Vehicles, %	0	0	0	0	2	0	3	3	0	0	0	0
Mvmt Flow	26	95	22	12	63	14	43	39	13	19	49	35
Major/Minor N	//ajor1		ı	Major2			Minor1		N	/linor2		
Conflicting Flow All	77	0	0	117	0	0	294	259	106	278	263	70
Stage 1	-	_	-	-	_	-	158	158	_	94	94	_
Stage 2	-	-	-	-	-	-	136	101	-	184	169	-
Critical Hdwy	4.1	-	-	4.1	-	-	6.73	6.13	6	8.3	7.7	6.8
Critical Hdwy Stg 1	-	-	-	-	-	-	5.73	5.13	-	7.3	6.7	-
Critical Hdwy Stg 2	-	-	-	-	-	-	5.73	5.13	-	7.3	6.7	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.527	4.027	3.3	3.5	4	3.3
Pot Cap-1 Maneuver	1535	-	-	1484	-	-	678	662	959	618	591	987
Stage 1	-	-	-	-	-	-	857	779	-	890	796	-
Stage 2	-	-	-	-	-	-	878	819	-	774	721	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1535	-	-	1484	-	-	599	645	959	570	576	987
Mov Cap-2 Maneuver	-	-	-	-	-	-	599	645	-	570	576	-
Stage 1	-	-	-	-	-	-	842	765	-	874	790	-
Stage 2		-	-	-	-	-	788	812	-	711	708	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	1.3			1			11.5			11.3		
HCM LOS							В			В		
Minor Lane/Major Mvm	t I	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR:	SBI n1			
Capacity (veh/h)		651	1535	-		1484	-	-	670			
HCM Lane V/C Ratio		0.145		_		0.008	_		0.154			
HCM Control Delay (s)		11.5	7.4	0		7.4	0	_	11.3			
HCM Lane LOS		В	Α	A	_	Α	A	_	В			
HCM 95th %tile Q(veh)		0.5	0.1	-	_	0	-	_	0.5			
		3.3	<b></b>						J.0			

Intersection												
Int Delay, s/veh	6.9											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	47	123	72	16	75	15	41	60	17	8	81	22
Future Vol, veh/h	47	123	72	16	75	15	41	60	17	8	81	22
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	-2	-	-	4	-	-	-2	-	-	6	-
Peak Hour Factor	89	89	89	89	89	89	89	89	89	89	89	89
Heavy Vehicles, %	0	0	3	0	2	0	8	0	0	13	0	0
Mvmt Flow	53	138	81	18	84	17	46	67	19	9	91	25
Major/Minor N	Major1		ا	Major2			Minor1		ľ	Minor2		
Conflicting Flow All	101	0	0	219	0	0	472	422	179	457	454	93
Stage 1	-	-	-	-	-	-	285	285	-	129	129	-
Stage 2	-	-	-	-	-	-	187	137	-	328	325	-
Critical Hdwy	4.1	-	-	4.1	-	-	6.78	6.1	6	8.43	7.7	6.8
Critical Hdwy Stg 1	-	-	-	-	-	-	5.78	5.1	-	7.43	6.7	-
Critical Hdwy Stg 2	-	-	-	-	-	-	5.78	5.1	-	7.43	6.7	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.572	4	3.3	3.617	4	3.3
Pot Cap-1 Maneuver	1504	-	-	1362	-	-	519	551	878	426	434	955
Stage 1	-	-	-	-	-	-	732	701	-	813	760	-
Stage 2	-	-	-	-	-	-	818	799	-	594	586	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1504	-	-	1362	-	-	403	521	878	360	411	955
Mov Cap-2 Maneuver	-	-	-	-	-	-	403	521	-	360	411	-
Stage 1	-	-	-	-	-	-	702	672	-	780	749	-
Stage 2	-	-	-	-	-	-	690	788	-	501	562	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	1.5			1.2			14.8			15.8		
HCM LOS							В			С		
Minor Lane/Major Mvm	t 1	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR :	SBLn1			
Capacity (veh/h)		499	1504	-	-	1362	-	-	458			
HCM Lane V/C Ratio		0.266		-		0.013	-	-	0.272			
HCM Control Delay (s)		14.8	7.5	0	-	7.7	0	-	15.8			
HCM Lane LOS		В	Α	Α	-	Α	Α	-	С			
HCM 95th %tile Q(veh)		1.1	0.1	-	-	0	-	-	1.1			

Intersection												
Int Delay, s/veh	5.9											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	25	92	22	11	64	13	41	37	12	18	46	34
Future Vol, veh/h	25	92	22	11	64	13	41	37	12	18	46	34
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	-2	-	-	4	-	-	-2	-	-	6	-
Peak Hour Factor	94	94	94	94	94	94	94	94	94	94	94	94
Heavy Vehicles, %	0	0	0	0	2	0	3	3	0	0	0	0
Mvmt Flow	27	98	23	12	68	14	44	39	13	19	49	36
Major/Minor N	Major1		ľ	Major2			Minor1		N	/linor2		
Conflicting Flow All	82	0	0	121	0	0	306	270	110	289	274	75
Stage 1	-	-	-	-	-	-	164	164	-	99	99	-
Stage 2	-	-	-	-	-	-	142	106	-	190	175	-
Critical Hdwy	4.1	-	-	4.1	-	-	6.73	6.13	6	8.3	7.7	6.8
Critical Hdwy Stg 1	-	-	-	-	-	-	5.73	5.13	-	7.3	6.7	-
Critical Hdwy Stg 2	-	-	-	-	-	-	5.73	5.13	-	7.3	6.7	-
Follow-up Hdwy	2.2	-	-	2.2	-	-	3.527	4.027	3.3	3.5	4	3.3
Pot Cap-1 Maneuver	1528	-	-	1479	-	-	667	654	955	606	581	980
Stage 1	-	-	-	-	-	-	851	775	-	883	790	-
Stage 2	-	-	-	-	-	-	872	815	-	766	715	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1528	-	-	1479	-	-	587	636	955	558	565	980
Mov Cap-2 Maneuver	-	-	-	-	-	-	587	636	-	558	565	-
Stage 1	-	-	-	-	-	-	835	760	-	866	783	-
Stage 2	-	-	-	-	-	-	780	808	-	703	701	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	1.3			0.9			11.6			11.5		
HCM LOS							В			В		
Minor Lane/Major Mvm	t N	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR :	SBI n1			
Capacity (veh/h)	<u> </u>	640	1528	-		1479	-	-	661			
HCM Lane V/C Ratio			0.017	_		0.008	_		0.158			
HCM Control Delay (s)		11.6	7.4	0	_	7.5	0	_	11.5			
HCM Lane LOS		В	Α	A	_	Α.5	A	_	В			
HCM 95th %tile Q(veh)		0.5	0.1	-	-	0	-	-	0.6			
		3.0	<b>J.</b> 1									

Intersection						
Int Delay, s/veh	0.3					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<b>1</b>	LDIK	1100	4	¥	, LOIK
Traffic Vol, veh/h	239	4	3	135	5	3
Future Vol, veh/h	239	4	3	135	5	3
Conflicting Peds, #/hr	239	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	riee -		riee -	None	Stop -	
Storage Length	-	NONE -	_	NOTIE	0	NONE -
Veh in Median Storage		-	_	0	0	-
	e, # 0 2			-2	0	
Grade, %		- 02	- 02			- 02
Peak Hour Factor	83	83	83	83	83	83
Heavy Vehicles, %	0	2	2	4	2	2
Mvmt Flow	288	5	4	163	6	4
Major/Minor	Major1	ı	Major2	N	Minor1	
Conflicting Flow All	0	0	293	0	462	291
Stage 1	-	-	-	_	291	-
Stage 2	-	_	_	-	171	_
Critical Hdwy	_	-	4.12	_	6.42	6.22
Critical Hdwy Stg 1	_	_		_	5.42	-
Critical Hdwy Stg 2	_	_	_	_	5.42	_
Follow-up Hdwy	_		2.218		3.518	
Pot Cap-1 Maneuver	-		1269		558	748
Stage 1	_		1209	-	759	740
Stage 2	_	_	-		859	-
	-		-	-	009	-
Platoon blocked, %	-	-	1000	-	EEC	740
Mov Cap-1 Maneuver		-	1269	-	556	748
Mov Cap-2 Maneuver		_	-	-	556	-
Stage 1	-	-	-	-	759	-
Stage 2	-	-	-	-	856	-
Approach	EB		WB		NB	
HCM Control Delay, s			0.2		10.9	
	U		0.2			
HCM LOS					В	
Minor Lane/Major Mvr	nt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		615	-	-	1269	-
HCM Lane V/C Ratio		0.016	-		0.003	-
HCM Control Delay (s	)	10.9	_	_	7.8	0
HCM Lane LOS	,	В	_	_	A	A
HCM 95th %tile Q(veh	1)	0	_	_	0	-
	.1					

Intersection						
Int Delay, s/veh	0.6					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<u>₽</u>	רטוע	TTDL	<del>યક</del>	¥	אטוו
Traffic Vol, veh/h	134	10	7	132	6	5
Future Vol, veh/h	134	10	7	132	6	5
Conflicting Peds, #/hr	0	0	0	0	0	0
· ·	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	_	-	-	-	0	-
Veh in Median Storage, #	# 0	-	-	0	0	-
Grade, %	2	-	-	-2	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	0	2	2	2	2	2
Mvmt Flow	149	11	8	147	7	6
Major/Minor Ma	ajor1		Major2		Minor1	
						155
Conflicting Flow All	0	0	160	0	318	155
Stage 1	-	-	-	-	155	-
Stage 2	-	-	4.40	-	163	-
Critical Hdwy	-	-	4.12	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	- 0.40	-	5.42	-
Follow-up Hdwy	-		2.218		3.518	
Pot Cap-1 Maneuver	-	-	1419	-	675	891
Stage 1	-	-	-	-	873	-
Stage 2	-	-	-	-	866	-
Platoon blocked, %	-	-	4440	-	074	004
Mov Cap-1 Maneuver	-	-	1419	-	671	891
Mov Cap-2 Maneuver	-	-	-	-	671	-
Stage 1	-	-	-	-	873	-
Stage 2	-	-	-	-	861	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.4		9.8	
HCM LOS			• • •		Α	
Min and an all Mailes Mannet		UDI 1	EDT	EDD	WDI	WDT
Minor Lane/Major Mvmt	ſ	VBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		756	-	-		-
HCM Lane V/C Ratio		0.016	-		0.005	-
						- 11
HCM Control Delay (s)		9.8	-	-	7.6	0
		9.8 A 0	-	-	7.6 A	A -